



**STRUCTURES
AND
ARCHITECTURE**

1st International Conference

Verification Processes for Cross Laminated Timber (CLT) in the Frame of EN 1995

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Speaker:

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holz bau forschungs gmbh, Graz

Guimarães, Portugal – 21-23 July 2010

- Introduction: Cross Laminated Timber
CLT – a Wood Based Product
- Bending under loads perp. to Plane
- Shear loads in Plane
- Conclusion and open points

Timber Solid Construction: Single family house „Dringel-Techt“



Timber Solid Construction: Single family house „Dringel-Techt“



Timber Solid Construction: Single family house „Dringel-Techt“



Timber Solid Construction: Single family house „Jeitler“



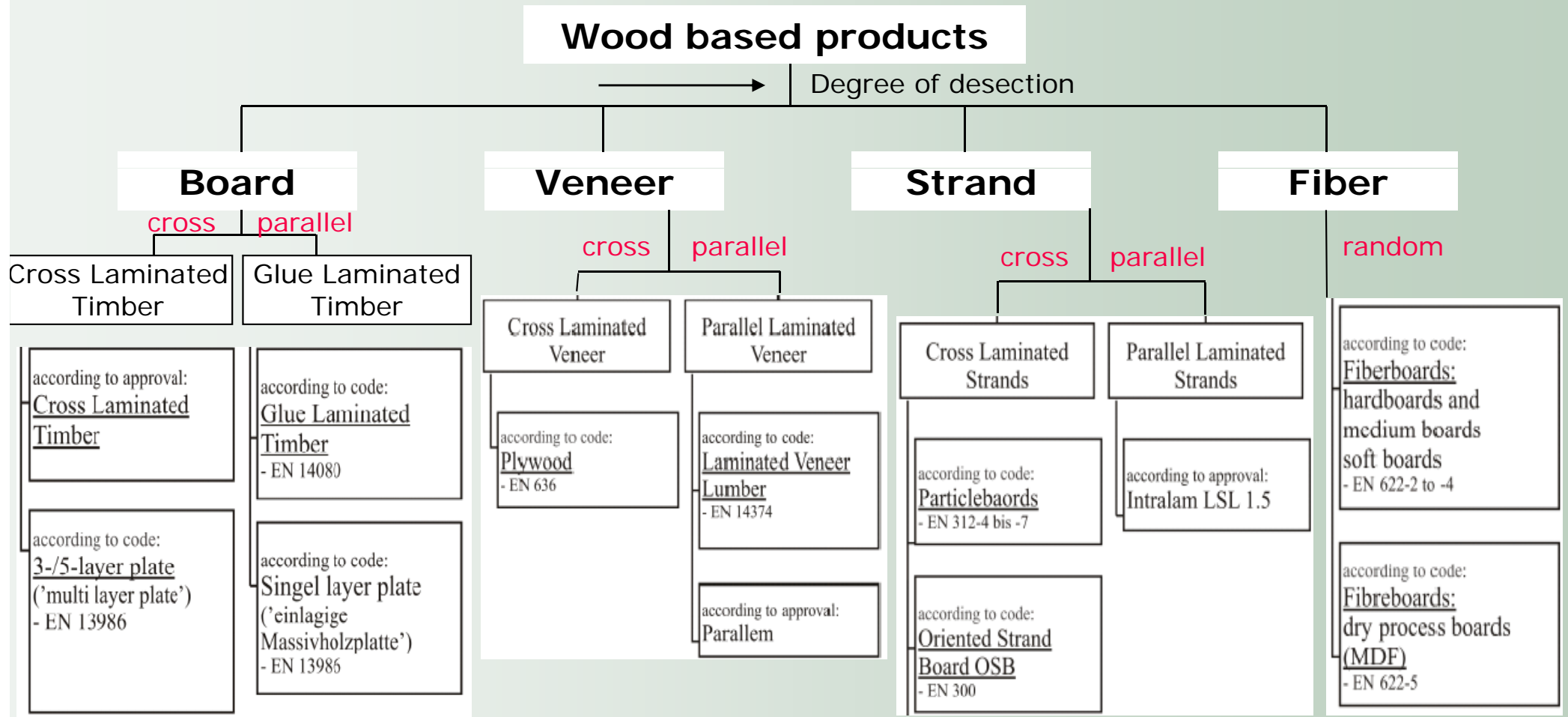
Timber Solid Construction: Wiskeymaker's house in St. Nikolai Styria/Austria



Foto-Quelle: Arch. G. Mitterberger, Graz

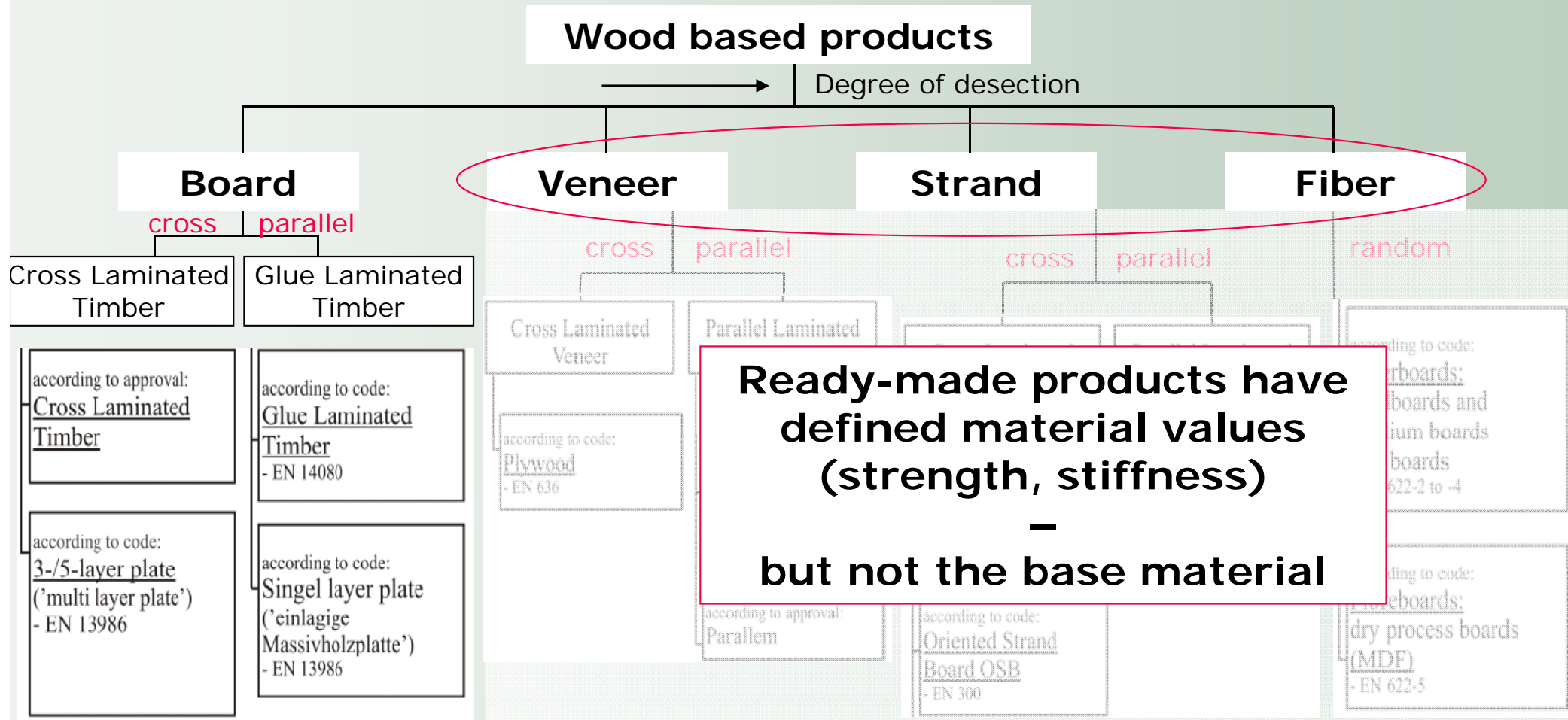
- Cross Laminated Timber CLT – a wood based product

One possibility of definition of wood based products:
degree of desection and orientation of base-material



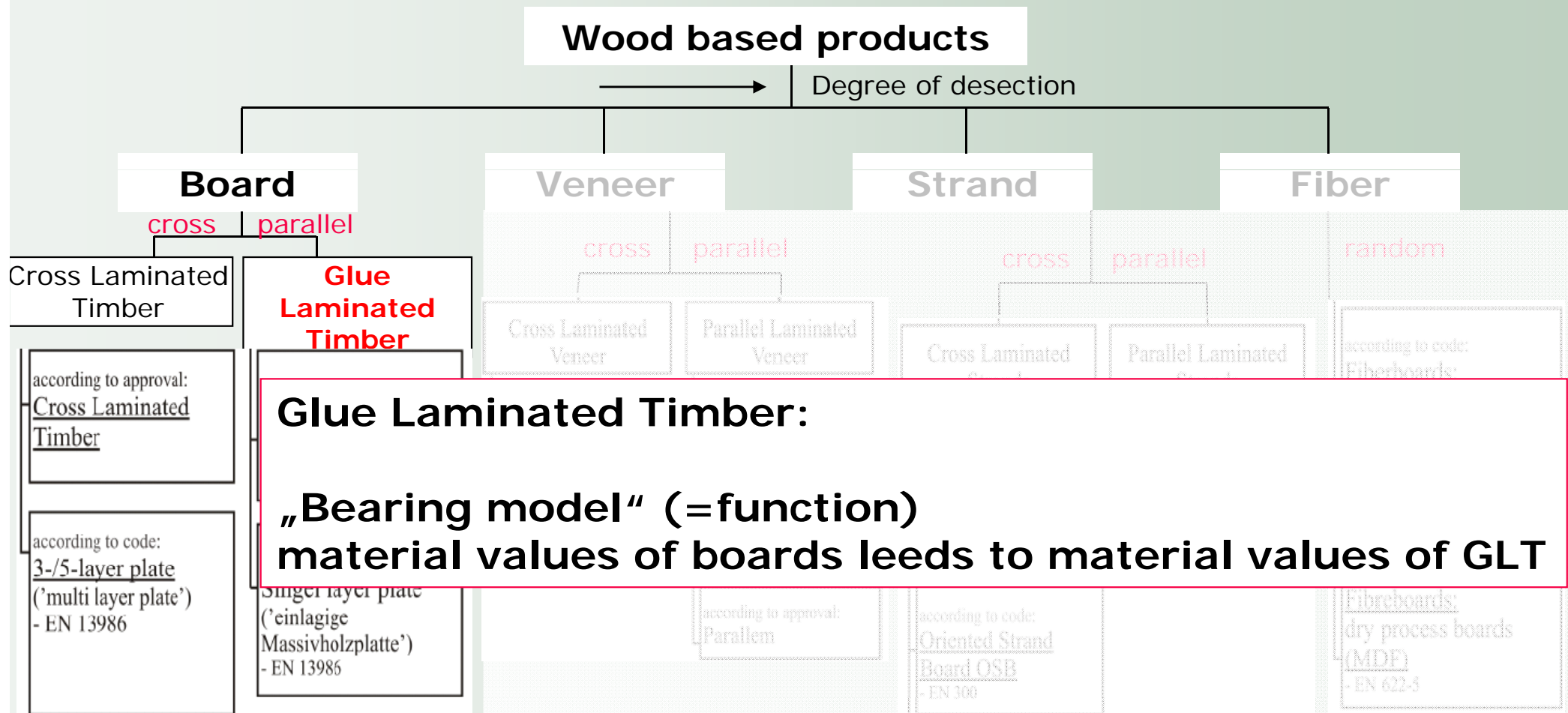
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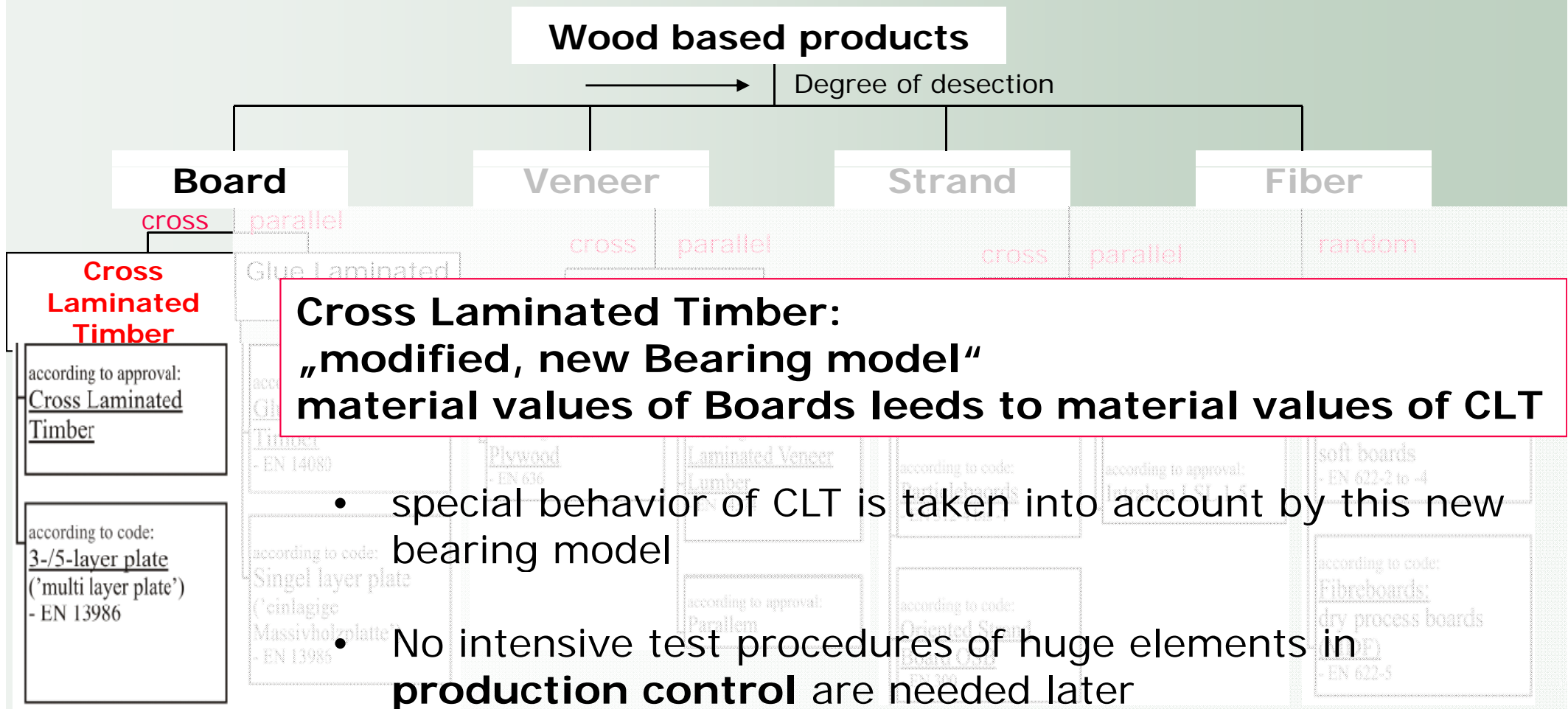
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- Cross Laminated Timber CLT – a wood based product

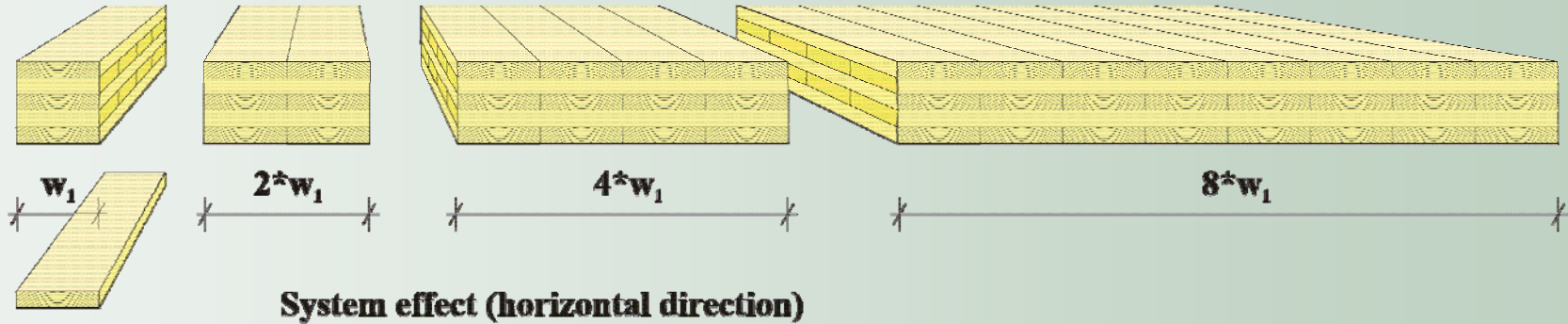
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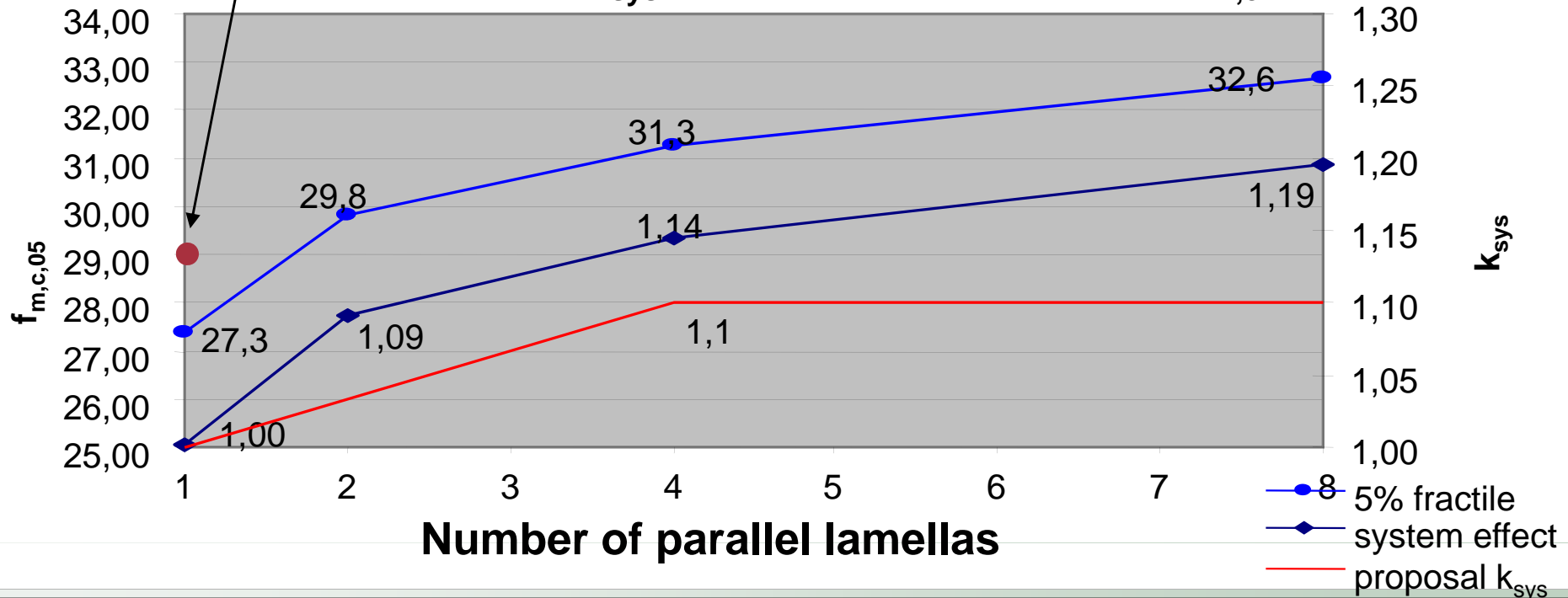
Bending in Plane – Bearing Model

Laminating effect (vertical direction)



19 # series ,1u' $f_{m,g,05} = 29,0 \text{ N/mm}^2$

5% fractile and k_{sys} of edge normal strength $f_{m,c}$



New proposal for GLT - Bearing Model:

$$f_{m,g,k} = a_{glt} \cdot f_{t,0,l,k}^{0,82} \quad \text{with} \quad a_{glt} = \begin{cases} 2,422 & COV_t = 0,25 \\ 2,811 & COV_t = 0,35 \end{cases}$$

$f_{m,g,k}$ bending strength of GLT

$f_{t,0,l,k}$ tensile strength of lamella

a_{glt} factor considering lamination effect for GLT

proposal for CLT - Bearing Model on the same basis:

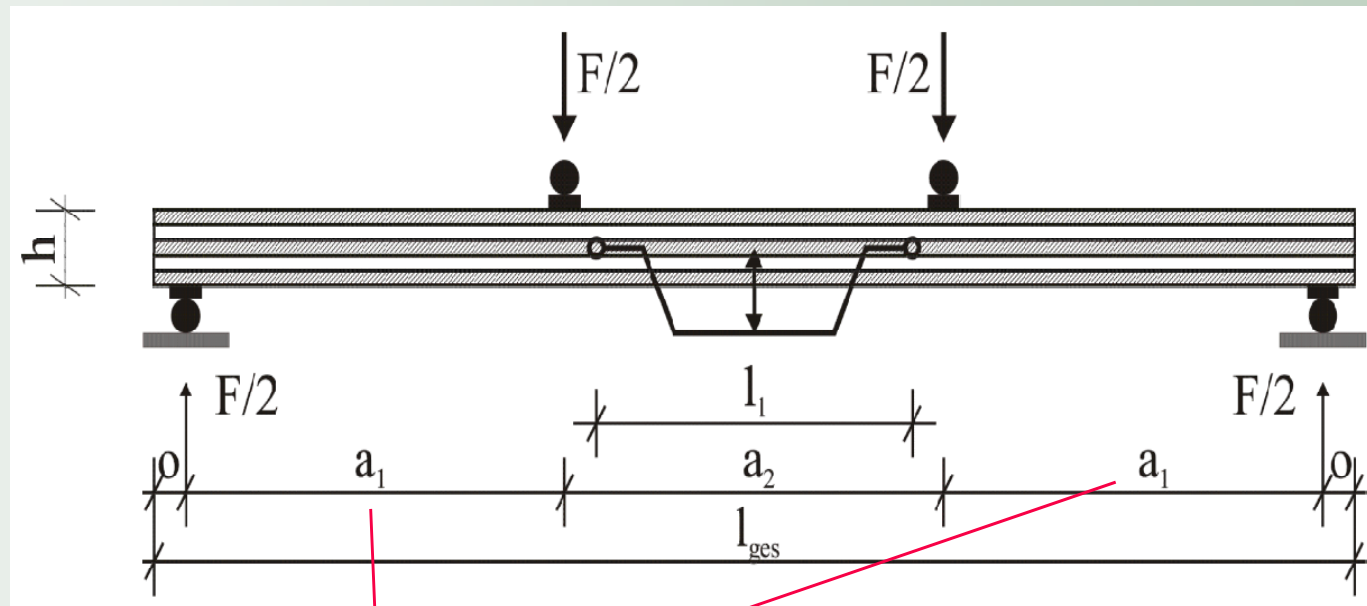
$$f_{m,e,k} = a_{clt} \cdot f_{t,0,l,k}^{0,80} \quad \text{with} \quad a_{clt} = \begin{cases} 3,0 & COV_t = 0,25 \\ 3,5 & COV_t = 0,35 \end{cases}$$

$f_{m,e,k}$ bending strength of CLT

$f_{t,0,l,k}$ tensile strength of lamella

a_{clt} factor considering system and lamination effect for CLT

4-point-bending test according to EN 408:



According to EN 408: $a_1 = f_{a1} \cdot h$ with $f_{a1} = 6 \pm 1,5$

4-point-bending test according to EN 408: **bending failure requested**

For bending failure we need at least a bending stress of 95%-quantile while (rolling) shear stress is 5%-quantile at a maximum

glulam

e.g.: GL 24 – visually graded → COV = 35%

$$\Rightarrow f_{m,c,k} = a \cdot f_{t0,l,k}^{0,80} = 2,811 \cdot 14^{0,82} = 24,5 \text{ N/mm}^2$$

CLT

$$\Rightarrow f_{m,c,k} = a_{\text{CLT}} \cdot f_{t0,l,k}^{0,80} = 3,5 \cdot 14^{0,8} = 28,9 \text{ N/mm}^2$$

95% quantile (assuming normal distribution):

$$f_{m,c,95} = \frac{f_{m,c,05}}{1 - 1,645 \cdot COV_m} \cdot (1 + 1,645 \cdot COV_m) = 40,3 \text{ N/mm}^2$$

4-point-bending test according to EN 408: **bending failure requested**

(Rolling) shear strength of cross layers:

Without groovings:

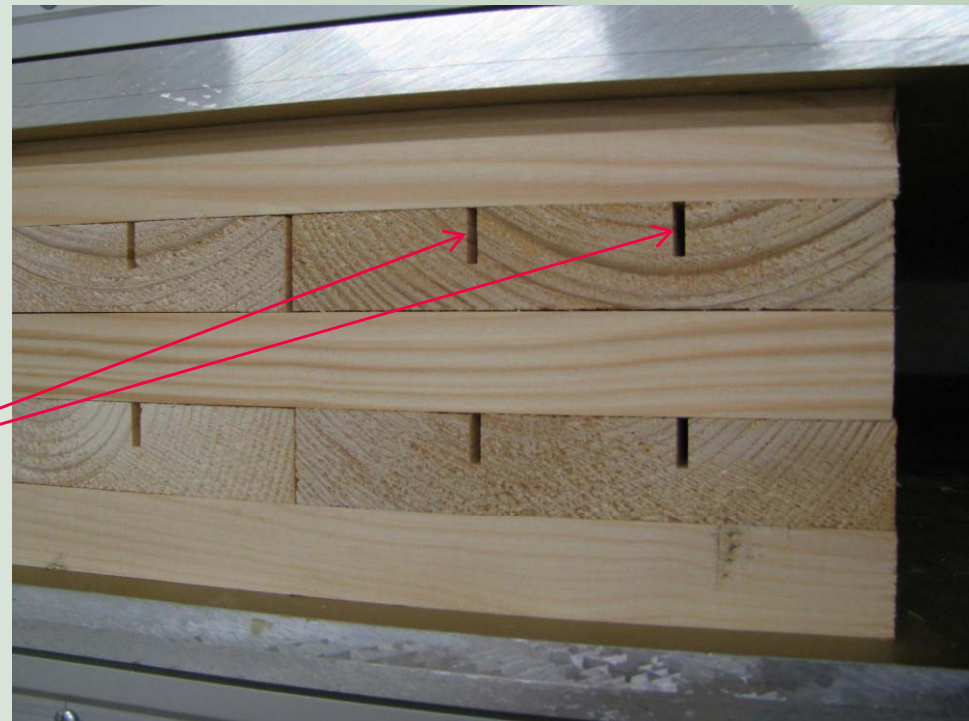
$$f_{v,9090,c,k} = 1,50 \text{ N/mm}^2$$

With groovings:

$$f_{v,9090,c,k} = 0,70 \text{ N/mm}^2$$

acc. to approval for Merk

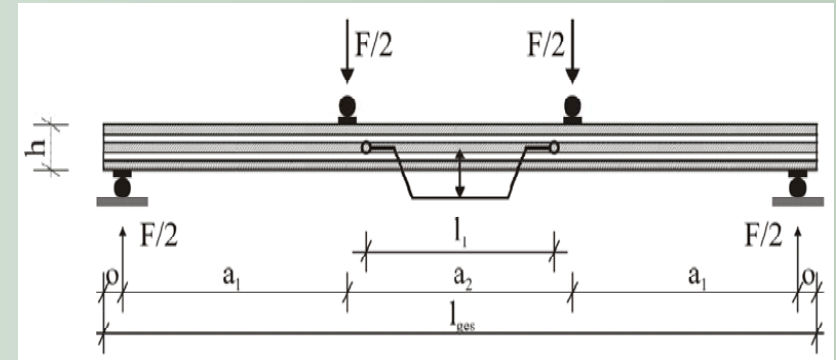
Example for
groovings



4-point-bending test according to EN 408: **bending failure requested**

$$\tau_{v,9090,c} = \frac{\frac{F}{2} \cdot S_{eff}}{I_{eff} \cdot b} \Rightarrow F = \frac{2 \cdot f_{v,9090,c,05} \cdot I_{eff} \cdot b}{S_{eff}}$$

$$\sigma_{m,c} = \frac{M}{W_{eff}} = \frac{\frac{F}{2} \cdot a_1}{W_{eff}} = \frac{F}{2} \cdot \frac{(f_{a1} \cdot h)}{W_{eff}} \Rightarrow f_{a1} = \frac{2 \cdot f_{m,c,95} \cdot W_{eff}}{F \cdot h}$$



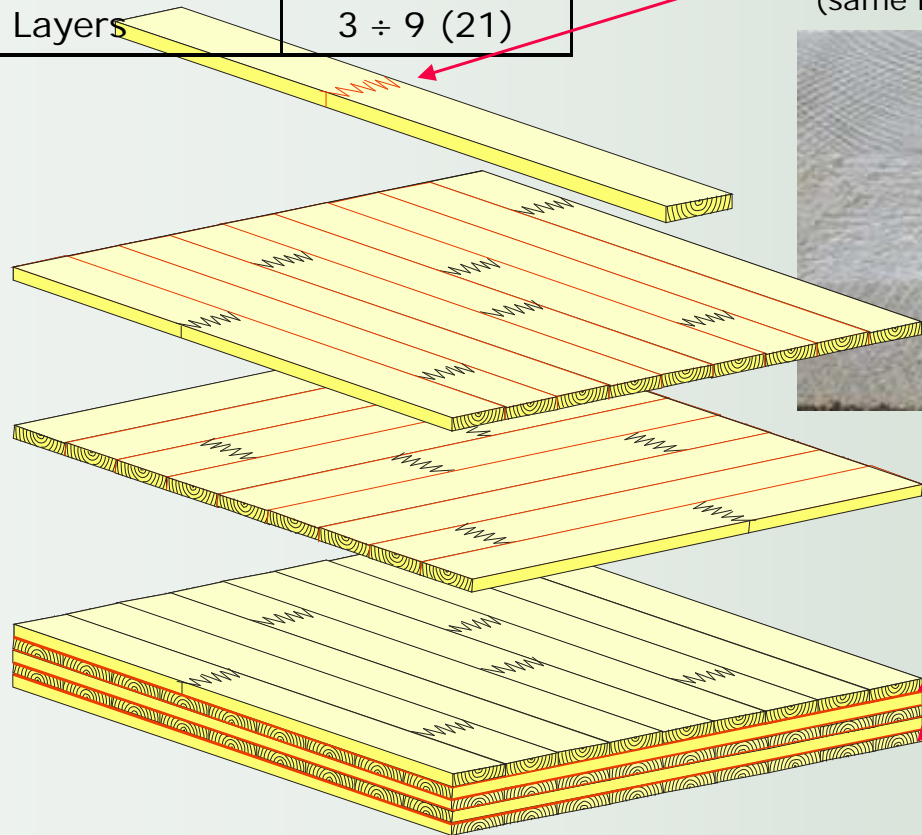
	unit	without grooving	with grooving
$f_{m,c,05}$	[N/mm ²]	28.9	28.9
$COV_{fm,c}$	[%]	10%	10%
$f_{m,c,95}$	[N/mm ²]	40.3	40.3
$f_{v,9090,c,k}$	[N/mm ²]	1.5	0.7
Faktor f_{a1}	[-]	6.0	12.8

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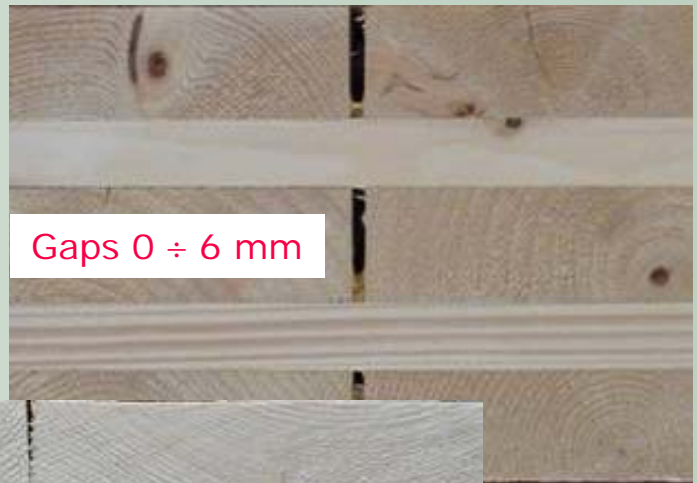
- Shear force in plane – Bearing Model

The mass product Cross Laminated Timber (CLT)

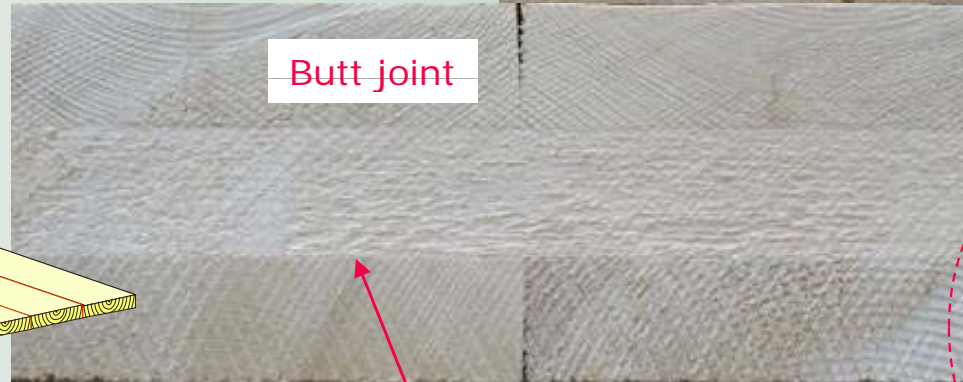
Length	16 m
Width	3 m
Thickness	60 ÷ 400 mm
Layers	3 ÷ 9 (21)



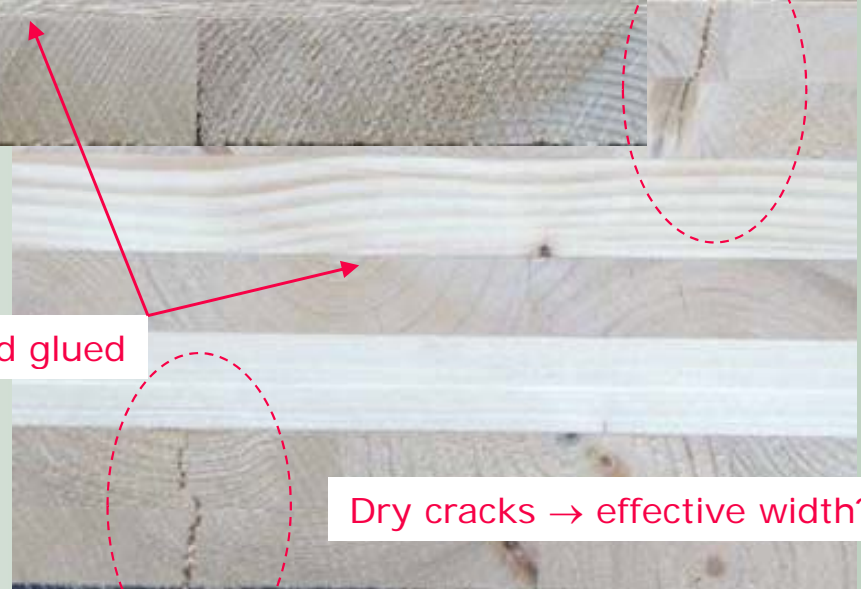
Finger Joint
Rigid glued
(same lamella as for GLT)



Gaps 0 ÷ 6 mm



Butt joint



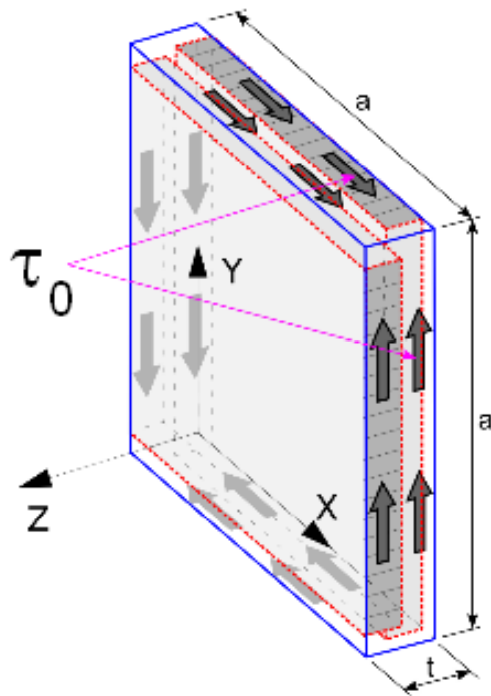
Rigid glued

Dry cracks → effective width?!

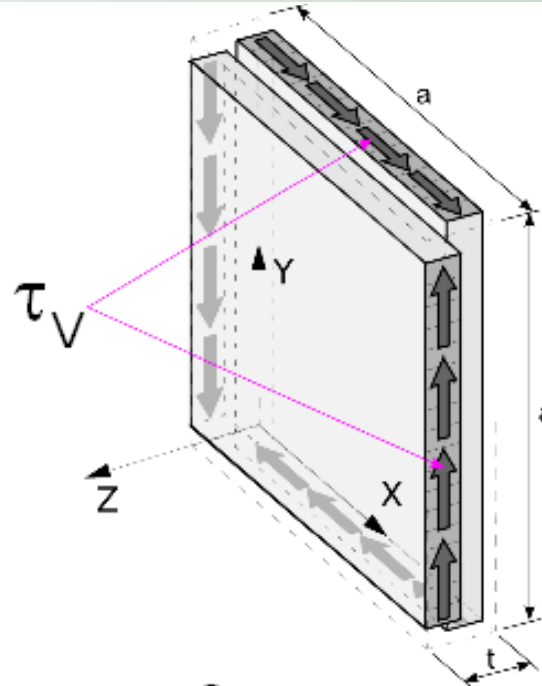
- Shear force in plane – Bearing Model

Mechanism I:

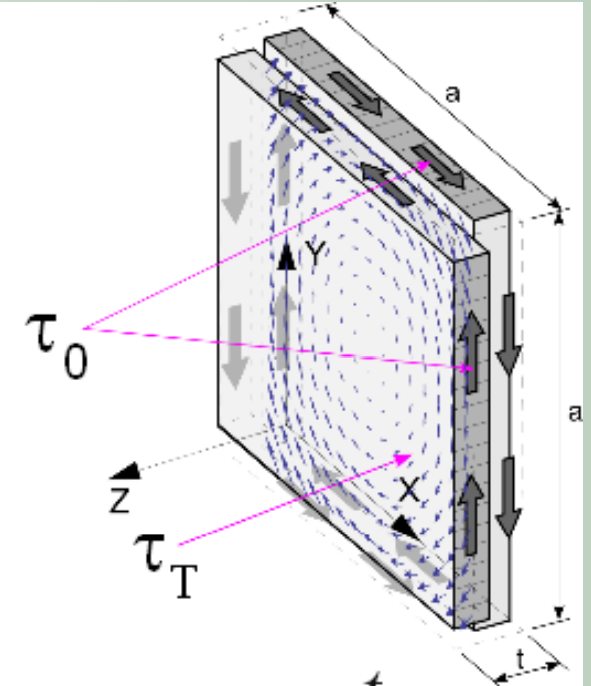
Mechanism II:



τ_0 nominal shear stress in plane



$\tau_V = 2 \cdot \tau_0$
 τ_V shear stress in cross section

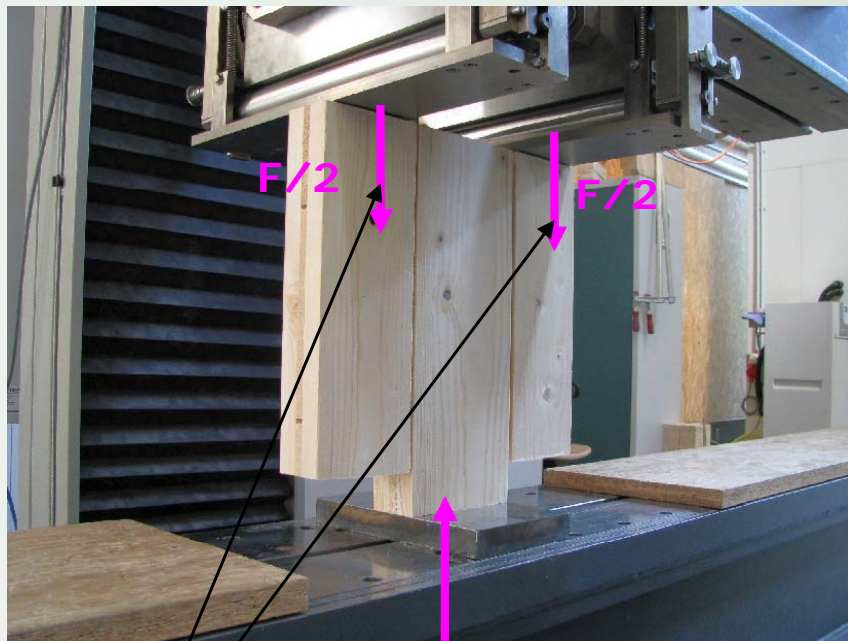


$\tau_T = 3 \cdot \tau_0 \cdot \frac{t}{a}$
 τ_T shear stress due to torsional moment in gluing interface

- Shear force in plane – Test Configuration

TUG test configuration – Mechanism I

Test configuration with symmetric shear block (2 tested cross sections)



Shear forces

Test configuration

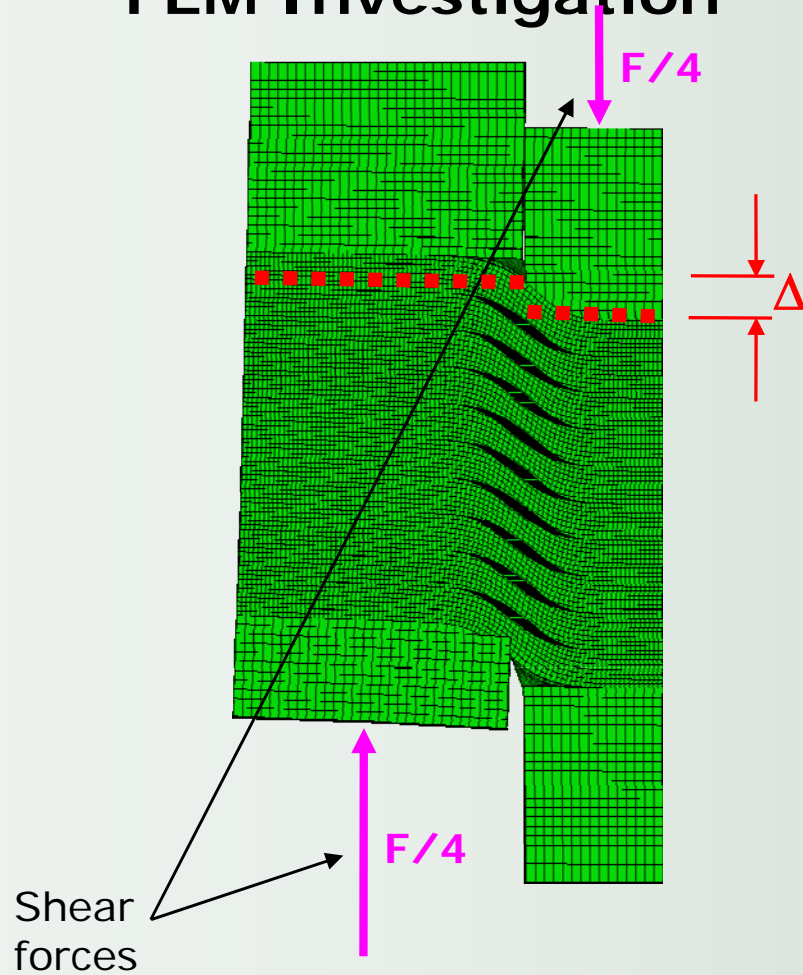


Local failure

- Shear force in plane – Test Configuration

TUG test configuration – Mechanism I

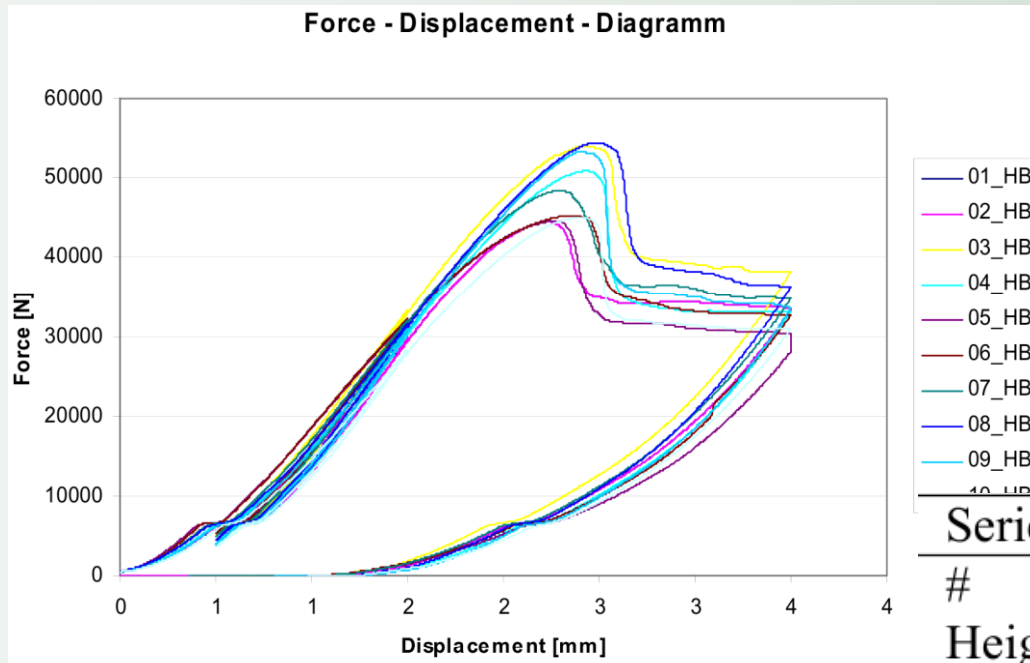
FEM Investigation



Local failure



- Shear force in plane – Test Configuration



Load — displacement behaviour

Shear Strength value for mechanism I

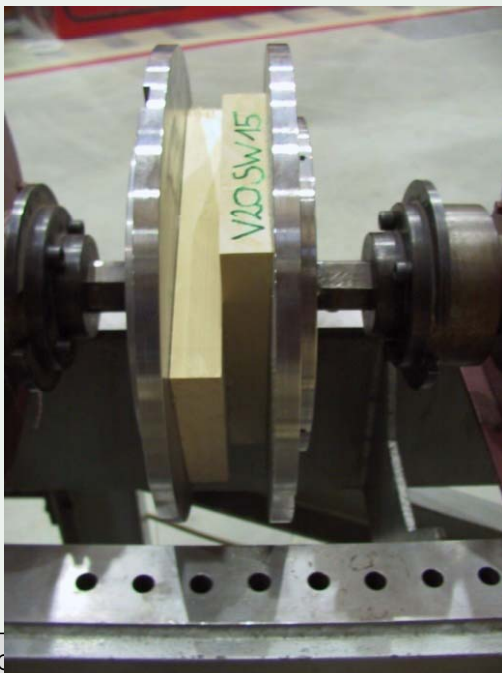
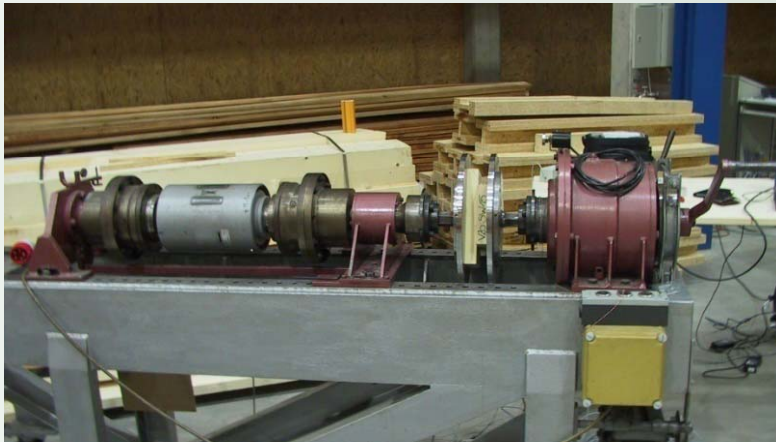
$$f_{v,k} = 10 \text{ [N/mm}^2\text{]}$$

Remark: Value still in discussion!

Series	Value	
#	20	[-]
Height a	200	[mm]
Thickness t	10	[mm]
$f_{v,d}$ - mean value	12.8	[N/mm ²]
Standard deviation	1.45	[N/mm ²]
COV	11.3%	[-]
$f_{v,d}$ 5% - Quantile normal distribution	10.4	[N/mm ²]
$f_{v,d}$ 5% - Quantile log normal distribution	10.6	[N/mm ²]
$f_{v,d}$ 5% - Quantile EN 14358	10.3	[N/mm ²]

- Shear force in plane – Test Configuration

TUG test configuration – Mechanism II shear stresses in the gluing interface



Detail of test specimen

$$\tau_{\max} = \frac{M_T}{I_P} \cdot \frac{1}{2} \cdot a$$

M_T Torsional moment
 I_P polar sectional moment of gluing interface

$$I_P = \frac{a^4}{6}$$

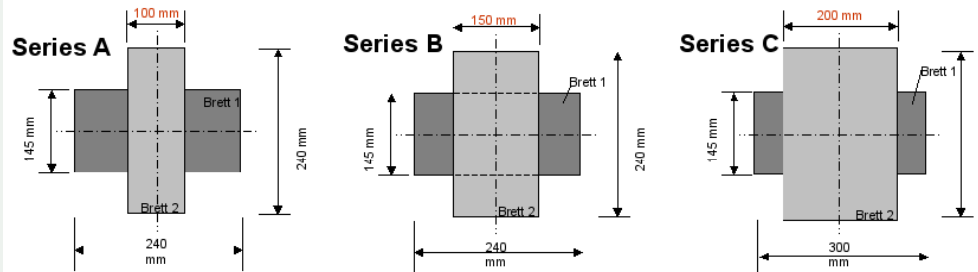
a dimension of RVE

2004: diploma thesis G. Jeitler
 „Versuchstechnische Ermittlung der Verdrehungs-
 kenngrossen von orthogonal verklebten Brettlamellen“

- Shear force in plane – Test Configuration

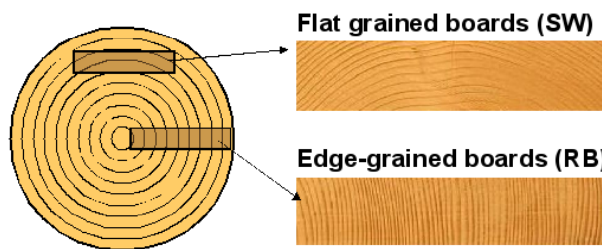
Coverage of tests

Variation of glued surface geometry



Annual ring gradient

Spruce (lat. picea abies)



Shear strength value for mechanism II

$$\tau_{\max} = \frac{M_T}{I_P} \cdot \frac{1}{2} \cdot a$$

Shear stresses in the gluing interface

Series	Annual ring orient.	5% Quantile	
A	Edge-grained	3.67	[N/mm ²]
A	Flat grained	2.79	[N/mm ²]
B	Edge-grained	3.20	[N/mm ²]
B	Flat grained	2.69	[N/mm ²]
C	Edge-grained	2.98	[N/mm ²]
C	Flat grained	3.10	[N/mm ²]

$$f_{T,k} = 2.5 \text{ [N/mm}^2\text{]}$$

Remark: Value generally accepted!

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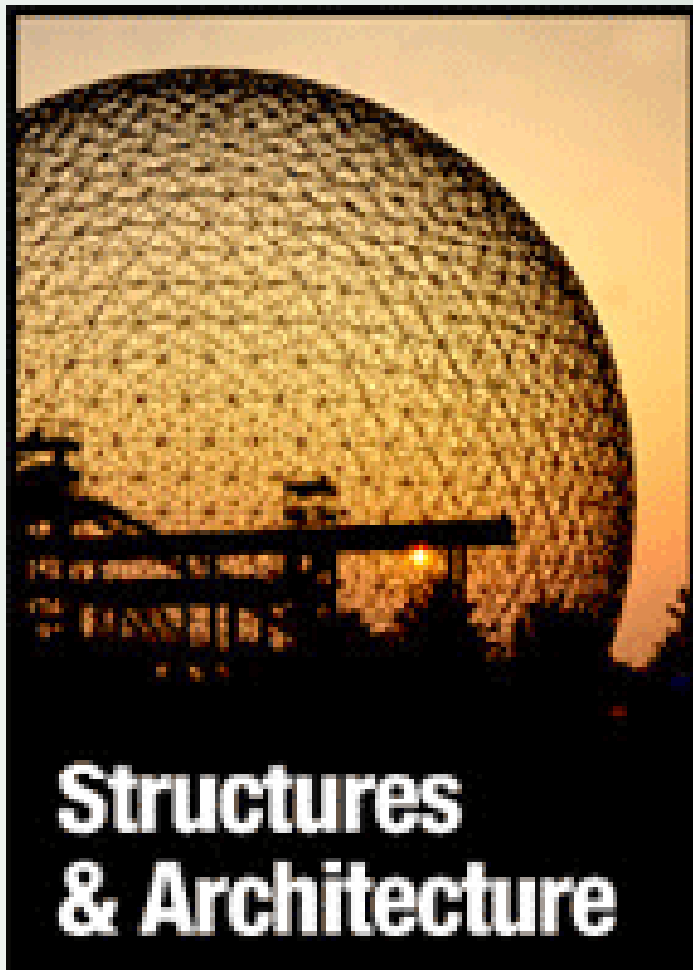
Conclusion

- Bearing models – like for bending loads perp. to plane or shear loads in plane - are needed for huge elements like CLT
- Test procedures must be new defined (shear loads in plane) or at least adapted (bending loads) from well known configurations (EN 408 configuration)

open points

- Bearings models for tension and compression forces should be still established.
- Interaction formulars between tension/compression with bending should be investigated in future.
- Bending strength near local support (e.g. column support) with its typical moment peaks has to be investigated.
- Compression strength perpendicular to grain – and factor $k_{c,90}$
- For some special cases tensile strength behavior perpendicular to grain is needed – e.g. curved elements.

Thank You for Your Attention



Contact:

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